# Bill Burton Fishing Pier Cambridge, Maryland Modified Underwater Inspection Report February 2022



For:

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03/26/2022

Date

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PROFESSIONAL CERTIFICATION:

"I hereby certify that this document was prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the State of Maryland, License No. 52301; Expiration Date: 02/14/2024."

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# **Table of Contents**

1	Intr	oduction	1
	1.1	Description of the Facility	1
	1.2	Inspection Procedures	1
	1.3	Condition Assessment Criteria	2
2	Obs	served Conditions	3
	2.1	Reinforced Concrete Pile Bents	3
	2.1.1	Reinforced Concrete Struts	3
	2.1.1	Square Reinforced Concrete Piles	4
	2.2	Pier Wall 1 (Bent 56)	5
	2.3	Pier Wall 6 (Bent 57)	5
	2.4	North Abutment (Bent 151)	6
3	Cor	nclusions and Recommendations	6
	3.1	Overall Condition Assessment Rating	6
	3.2	Recommendations	6
	3.2	1 Immediate (Within 6 months)	6
	3.2	2 Priority (Within 2 years)	7
	3.2	3 Routine (Within 5 years)	7

# Appendices

Appendix A: Inspection Limits and Reference Drawings

Appendix B: Photos

Appendix C: ASCE Damage Grading Figure



# 1 Introduction

Marine Solutions, Inc. (Marine Solutions) was retained by EBA Engineering, Inc. (EBA Engineering) to perform a modified underwater inspection of the substructure units that comprise the Bill Burton Fishing Pier near Cambridge, Maryland. The modified underwater inspection was to perform a Level I Inspection of 32 bents out of the 151 bents (approximately 20%), complete a Level II inspection (cleaning three 12" high bands near the splash zone, mid-height, and near mudline) on 32 piles (approximately 20%), and document water depth measurements at both ends of each bent during the dates of February 22<sup>nd</sup> through the 24<sup>th</sup> 2022. The purpose of the inspection was to document existing structural conditions, assess the overall site conditions, and provide recommendations for future actions. This report includes a description of the facility. inspection procedures. condition assessment criteria, observed conditions. and recommendations for repairs.

Previous inspection reports and facility plans were available and provided by EBA Engineering and used for comparison during the modified underwater inspection. For reporting purposes, bents and piles were numbered from the south and the east, respectively.

## **1.1 Description of the Facility**

The Bill Burton Fishing Pier was originally constructed in 1935 as the Emerson C. Harrington Bridge. The original bridge was approximately two miles long spanning across the Choptank River connecting the Maryland towns of Trappe and Cambridge. Due to increased traffic demands, the four-lane Frederick C. Mulkus Bridge was constructed in 1987 adjacent to the original bridge and became the current Route 50 bridge. The draw bridge section was removed from the original bridge to allow vessel traffic to pass through. The structure was turned into a pedestrian fishing pier and later named after Bill Burton, a local fisherman, writer, and reporter.

The Bill Burton Fishing Pier is currently comprised of two separate structures. The south structure is approximately 3,000' long and is supported by 56 bents (Photo 1). The south pier is approximately 5,000' long and supported by 95 bents (Photo 2). The bents are typically comprised of 5 square reinforced concrete piles with a horizontal concrete strut that encompasses all five piles (Photos 3 and 4). The struts are located 4' below the top of the piles within the tidal zone. Several bents appear to have had the strut removed and pile jackets installed (Photo 5). Approximately every 8 bents consist of a 10-pile double bent with a reinforced concrete strut (Photo 6).

Pier Wall 1 (Bent 56) is the northern most bent on the south structure and consists of two reinforced concrete columns orientated east and west (Photo 7). The columns are connected by a full-height reinforced concrete web wall. The columns and the web wall are founded on a reinforced concrete footing and subfooting. Pier Wall 6 (Bent 57) is the southernmost bent on the north structure and consists of two reinforced concrete columns orientated east and west (Photo 8). The columns are connected by a partial height reinforced concrete strut that extends from 7' below the high-water mark down to the top of the footing. The columns are founded on a footing and subfooting. The North Abutment (Bent 151) is comprised of a reinforced concrete stem, two reinforced concrete wingwalls, and a footing (Photo 9). The South Abutment was dry and did not require an underwater inspection.

## **1.2 Inspection Procedures**

The underwater inspection was performed by a three-person dive team lead by a Professional Engineer/Dive Supervisor. Diving operations were conducted using surface-supplied diving equipment with hardwire communications between the diver and topside personnel, and in accordance with all applicable ADCI, OSHA, and USCG regulations. The operations were staged from a fully equipped diving support vessel (DSV). All Marine Solutions on-site personnel were



experienced in the inspection, maintenance, rehabilitation, and construction of waterfront structures.

The inspection was performed in general accordance with ASCE Manuals and Reports on Engineering Practice No. 130: *Waterfront Facilities Inspection and Assessment* (ASCE Manual). The purpose of the inspection was to document existing structural conditions, assess the overall site conditions, and provide recommendations for future actions.

The underwater inspection included 35 randomly selected bents which included Pier Wall 1, Pier Wall 6, and the North Abutment. A Level I inspection effort (visual/tactile) was performed on 100% of the accessible structural elements within the selected bents from the high-water mark down to the mudline. A Level II inspection effort (visual inspection of cleaned areas) was performed on approximately 20% of the accessible structural elements within the selected bents. Water depths measurements were recorded at the east and west ends of each selected bents using a diver carried depth gauge. See Appendix A for detailed inspection notes and water depth measurements.

## **1.3 Condition Assessment Criteria**

In accordance with ASCE, the inspection condition and assessment criteria use a six-point standardized rating system provided in the ASCE Manual. The condition assessment of the facility is determined based on the findings during the underwater inspection. These ratings are required to categorize the results of the inspection and to provide a basis for comparison of new deficiencies in future inspections or other facilities. The condition assessment ratings as defined in the ASCE manual are described in Table 1 below.



Table 1: ASCE Condition Assessment Rating	ıs.
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Rating	Description
6 – Good	No visible damage or only minor damage noted. Structural elements may show very minor deterioration, but no overstressing observed. No repairs are required.
5 – Satisfactory	Limited minor to moderate defects or deterioration observed but no overstressing observed. No repairs are required.
4 – Fair	All primary structural elements are sound but minor to moderate defects or deterioration observed. Localized areas of moderate to major deterioration may be present but do not significantly reduce the load-bearing capacity of the structure. Repairs are recommended, but the priority of the recommended repairs is low.
3 – Poor	Advanced deterioration or overstressing observed on widespread portions of the structure but does not significantly reduce the load-bearing capacity of the structure. Repairs may need to be carried out with moderate urgency.
2 – Serious	Advanced deterioration, overstressing, or breakage may have significantly affected the load-bearing capacity of primary structural components. Local failures are possible, and loading restrictions may be necessary. Repairs may need to be carried out on a high-priority basis with urgency.
1 - Critical	Very advanced deterioration, overstressing, or breakage has resulted in localized failure(s) of primary structural components. More widespread failures are possible or likely to occur, and loading restrictions should be implemented as necessary. Repairs may need to be carried out on a very high-priority basis with strong urgency.

Each structural element was evaluated and assessed an element level damage rating (minor, moderate, major, or severe). See Appendix C for the ASCE Manual Element Level Damage Rating Figure. The element damage ratings were utilized to give an overall condition rating to each structural element inspected based on the marine environment, overall use, redundancy, section loss, bearing capacity, physical damage, and deterioration of each individual element.

# 2 Observed Conditions

The observed conditions of each structural element inspected at the Bill Burton Fishing Pier is discussed in the following sections. Please refer to Appendix A for Field Note Data detailing the exact locations, type, and extend of the observed defects. Refer to Appendix B for photographs of observed defects. Refer to Appendix C for ASCE Element Level Damage Rating Figure.

## 2.1 Reinforced Concrete Pile Bents

Marine Solutions inspected 27 reinforced concrete five pile bents and 5 reinforced concrete ten pile bents. Seven out of the 32 reinforced concrete bents did not have reinforced concrete struts. The reinforced concrete piles and struts are discussed in more detail in the sections below.

## 2.1.1 Reinforced Concrete Struts

The reinforced concrete struts are in overall **Poor** condition. The north and south faces of the struts typically have severe spalling that extends full length x full height and up to 10" deep (full-



depth) with the deepest areas of spalling concentrated in a 2' high band at the bottom of the strut (Photos 10 and 11). The spalling exposes all the main longitudinal bars and stirrups, which exhibit severe corrosion and section loss. The exposed stirrups typically are debonded and have areas of 100% section loss. The exposed longitudinal reinforcing bars are intermittently debonded and have 50% section loss throughout with isolated areas of up to 90% section loss. Areas without spalling typically have corrosion cracks with rust staining up to 1/4" wide with associated delamination.

The east and west faces of the struts typically have severe spalling that extends full width x full height and up to 20" deep (full-depth) with the deepest areas of spalling concentrated within the top and bottom 1' of the strut (Photos 12 through15). The spalling exposes several horizontal and vertical steel reinforcing bars which exhibit severe corrosion and section loss. The exposed reinforcing bars are intermittently debonded in random areas and have up to 100% section loss. Areas without spalling typically have corrosion cracks with rust staining up to 1/4" wide with associated delamination.

## 2.1.1 Square Reinforced Concrete Piles

The square reinforced concrete piles are in overall **Fair** condition. This is due to widespread minor to moderate abrasion, spalling, and cracking (Photos 16 through 19). Most of these defects are located within the top 4' of the pile above the reinforced concrete strut. Piles that do not have a strut or repairs in the tidal zone typically have abrasion up to 2" deep most notably at the corners. Bent 148, Pile 4 has an area of abrasion with exposed reinforcement where the strut is missing due to deterioration (Photo 20). Approximately 18% of the piles have corner spalls within the top 5' of the piles (Photos 21). The spalls are up to 5'-0" high x up to 10" wide x up to 7" deep with exposed reinforcement that exhibits 50% to 75% section loss. Three piles (Bent 90 Pile 1 and 5; Bent 149 Pile 5) have several spalling and deterioration from 3' to 7' below the top of the piles (Photo 22). These spalls are approximately 4'-0" long x full-width x 7" deep with exposed and corroded reinforcement with up to 50% section loss. Approximately 75% of the piles have cracking typically 1/8" to 1/4" wide with isolated 1/2" wide cracks within the top 4' of the pile. As a result of the cracking, there are areas of associated delamination and spalling on all faces of the affected piles (Photos 18 through 22). See Table 3 for total quantities and percentages of piles with observed defects.

Defect Type	No. of Piles	Percentage of Piles with Defect
Abrasion	15	8%
Spall	34	18%
Cracking	138	75%
< 1/8" Wide	15	8%
1/8" to 1/4" Wide	112	61%
>=3/8" Wide	11	6%
With Spalling and Delamination	36	19%

Table 2. Type and Number of Pile Defects	Table 2:	Type an	d Number	of Pile	Defects.
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Approximately 92% of the piles have been previously repaired with either a square epoxy filled fiberglass jacket or a grout filled fabric bag with welded wire mesh. Piles within bents without a strut typically have the square epoxy filled fiberglass jackets. These jacket repairs typically extend



from 3' to 10' below the top of the pile (Photo 23). Isolated piles with these repairs have areas of missing or damaged epoxy epoxy jackets typically from 4' to 9' below the top of the pile (Photos 24 and 25). Piles within bents with a strut typically have the grout filled fabric bag repairs. The bag repairs typically extend from the bottom of the strut to 13' below the top of the pile. Isolated bag repairs have voids up to 1'-0' high x 8" wide x 2" deep exposing the wire mesh within the grout (Photo 26). At these locations the underlying pile is not exposed. For specific repair numbers and percentages, see Table 3.

Repair Type	No. of Piles	No. of Pile with Damage/Missing Area	Percentage of Total Piles Inspected	Percentage of Total Repairs with Defects
No Repair	15	0	8%	0%
Square Epoxy Jacket	34	5	18%	15%
Grout Filled Fabric Bag	136	3	74%	2%
Totals:	185	8	100%	4%

Table 3 <sup>.</sup> Number	of Pile Repair	s and Defects
		S and Derects.

# 2.2 Pier Wall 1 (Bent 56)

Pier Wall 1 (Bent 56) is in **Poor** condition. The west column exhibits random areas of scale up to full height x 3" deep. Full circumference of the footing/column interface, there is spalling up to 10'-0" wide x 2'-6" high x 6" deep with exposed and corroded reinforcement. The east column exhibits random areas of scale up to full height x 3" deep. Full circumference of the footing/column interface, there is spalling up to 10'-0" wide x 3'-0" high x 10" deep with exposed and corroded reinforcement.

The web wall exhibits random areas of scaling 1" deep and isolated areas 9" deep concentrated at the waterline (Photo 27). The top of the footing is approximately 5'-6" below the waterline and approximately 8'-0" high. There are random areas of scaling up to 8" deep throughout with exposed and loose aggregate. The footing has large areas of latent concrete throughout resulting in small voids and removal of the concrete matrix when sounded with a hammer. At the west face near the centerline, there is an area of undermining 3'-0" long x 1'-6" high x 2'-0" deep.

# 2.3 Pier Wall 6 (Bent 57)

Pier Wall 6 (Bent 57) is in **Poor** condition. The west and east columns have 9'-0" high grout filled fiberglass jackets that begin 4' above the waterline and extend within 4" to the top of the strut. In the tidal zone, there are random areas of missing jacket throughout exposing the underlying grout layer. The grout exhibits 3" deep scale; however, is not deep enough to reach the original column This deterioration is most notable on the west faces of both columns (Photo 28).

The top of the reinforced concrete strut is located approximately 7' below the high water mark and has random areas of voids and spalling up to 1'-0" diameter x 3" deep (Photo 29). The top of the footing is located approximately 15' below the high-water mark. The footing is 5'-0" high x 4'-0" wide and encompasses both columns. The footing has large areas of latent concrete throughout resulting in small voids and removal of the concrete matrix when sounded with a hammer. There are random up to 1'-0" diameter x 3" deep voids throughout (Photos 30 and 31).



The top of the subfooting is located approximately 20' below the high-water mark. The subfooting is exposed up to 7'-0" x 2'-0" wide and encompasses the entire footing. The subfooting has large areas of latent concrete throughout resulting in small voids and removal of the concrete matrix when sounded with a hammer. There are random up to 5'-0" diameter x 1'-6" deep voids throughout (Photo 32). No signs of undermining were observed during the inspection.

# 2.4 North Abutment (Bent 151)

The North Abutment (Bent 151) is in **Fair** condition. The footing of the North Abutment and both wingwalls is exposed full-length x up to 3'-0" high with no signs of undermining and up to 3" deep scale. At the centerline of the stem wall and extending to the footing, there is a 6'-0" high x 2'-0" wide x 2" deep spall. At the east end of the stem wall, there is a spall of similar size. At the west end of the stem wall, there is a 2'-0" high x 1'-0" wide x 3" deep spall (Photos 33 and 34). At the interface of the stem wall and Northwest Wingwall near the waterline, there is a 2'-0" high x 1'-0" wide x 2" deep spall.

# **3** Conclusions and Recommendations

The Bill Burton Fishing Pier was assigned a combined overall Condition Assessment Rating (CAR). The CAR is assigned based on the type and level of severity of the deterioration observed on the individual structural components of each structure. Recommendations are provided based on the facilities current use, redundancy of structural elements, known history and age of the facility, and based on the areas inspected during this modified underwater inspection.

# 3.1 Overall Condition Assessment Rating

The Bill Burton Fishing Pier is in overall **Poor** condition. This CAR is given due to widespread areas of minor to moderate deterioration and isolated areas of major to severe deterioration on the primary structural elements.

## 3.2 Recommendations

Marine Solutions has developed recommendations for additional actions and rehabilitation to the various structures or areas at the Bill Burton Fishing Pier based on observed conditions. The following recommendations should be compared with facility use and production parameters to determine the most practical and economical allocation of funds.

Recommendations have been separated by immediate, priority, and routine repair items. It is recommended that immediate repair items be performed as soon as possible within the next six months. Immediate recommendations typically include action items to increase the facilities overall safety or to address significant structural concerns. Priority repair items should be executed within the next two years to minimize the potential for facility limitations or load restrictions. Typically, structural components which are observed with major to severe deterioration are included in the priority repair category due to potential loss of capacity to the element. Routine repair items are maintenance items which should be executed within the next five years. Routine maintenance items help extend the service life of the facilities and minimize the need for structural repairs and rehabilitation which are often costly and can involve temporary facility shutdowns.

## 3.2.1 Immediate (Within 6 months)

Based on the inspection limits and observed conditions at the time of inspection, Marine Solutions recommends the following repair items on an immediate basis:

• Due to the age of the structure and the overall CAR of poor, the Bill Burton Fishing Pier should have a complete underwater inspection of the remaining piers not included within this inspection.



- Perform a load rating analysis to determine if the current loading criteria requires the reinforced concrete struts to provide adequate structural capacity to the piers. If the struts are required, then they should be repaired on all bents where they remain. If they are not required, then they should be removed, and the underlying pile section should be inspected.
- Even though the struts are in overall poor condition and several struts have severe deterioration, some struts only had moderate to major deterioration and act as a protective element around the piles in the tidal zone. If the load rating determines the struts are not required for structural capacity, then struts with moderate to major damage could be left in place.
- Three piles have severe deterioration with exposed and debonded main reinforcing members. These piles should be repaired with new reinforcing bars and a grout filled structural jacket.

#### 3.2.2 Priority (Within 2 years)

Based on the inspection limits and the observed conditions at the time of inspection, Marine Solutions recommends the following repair items on a priority basis:

- Replace all damaged or missing square epoxy filled fiberglass jackets with new grout filled fiberglass jackets.
- At Pier Wall 1 (Bent 56), there is an area of undermining. Backfill and place scour protection countermeasures to prevent further undermining.
- Remove areas of unsound concrete, clean corroded reinforcement, and patch the areas of deep (>1" deep) spalling on the piles, pier walls, and North Abutment (Bent 151).
- At Pier Wall 1 (Bent 56) and Pier Wall 6 (Bent 57), repair the areas of voids in the footing and subfooting.

## 3.2.3 Routine (Within 5 years)

Based on the inspection limits and observed conditions at the time of inspection, Marine Solutions recommends the following repair items on a routine basis:

- Epoxy seal all cracks equal to and greater than 1/8" wide on the piles.
- Continue to monitor the condition of the Bill Burton Fishing Pier by maintaining a consistent underwater inspection cycle.



Appendix A – Field Inspection Notes

## Abbreviations Used in the Field Inspection Notes:

N – North S - South W-West E – East NE – Northeast NW – Northwest SW – Southwest SE - Southeast SEC – Southeast Corner SWC – Southwest Corner NWC – Northwest Corner NEC – Northeast Corner SL – Section Loss FL – Full-Length FW – Full Width FD – Full Depth FH – Full Height ML – Mudline CRX – Cracks HL – Hairline ISO - Isolated B1 – Bent Number 1 P1 – Pile Number 1

		nventory D	ata			Leve Dat	a									Defect Co	nd t on Data											
		intentory bi		F F			-		Abras	on Sca e				Spa				١	/ert ca Crack	ng			Ex s trig	g Repa rs		Manua		
Ben No	D PeNo	o Top o WL	Ben ML Deph()	Ben ML Deph()	Org X sec (n)	M n X Sec (n)	ML Dep h (	) Deph(n	) Delec Top	De ec Boom	Exp Stee	Dep h (n)	Face	De ec Top	De ec Boom	Exp Stee	W d h (n)	Face	De ec Top	De ec Boom	Rus Stan	Yes No	Repar Top	Repar Boom	Repar Type	Damage Ra ng	Concrete Strut Notes	Add t ona Notes
			WI TO MI	WI TO MI																								
	1	5		3				1	6	ML	No						3/16	S,E	0	4	Yes							6' TO ML SCALE ON CORNERS 1" DEEP
	2	5			22	18	2	2	6	8	No																	SCALE 1" DEEP ON CORNERS OF PILE
	3	5						1.5	6	8	No						>= 1/2	SEC, SWC	0	4	Yes	1					SOUTH FACE: FULL LENGTH STIRRUP	
8	4	5						1	6	8	No																EXPOSED, TOP AND BOTTOM REINF: EXPOSED. STIRRUPS 100% SL AT BOTTOM; SCALE UP TO 6" DEEP; NORTH FACE:	SOUTH AND EAST FACES P 0 TO 4 HEAVY DELA AND IMMINENT SPALLS WITH INTERMITTENT
	5	5						1.5	6	8	No						3/16	SEC, NWC	0	4	Yes						SPALLING FL X FH X 6" DEEP, STIRRUPS 100% SL OTHER REINE 50% SL	SCALE 1" DEEP ON CORNERS
	6	5						1.5	6	8	No	3.5	NEC	0	5	Yes w/ SL											SE, OTHER REINF: 30% SE	5'H X 2' W X 3.5" D SPALL, REINF. 75% SL
	7	5						1.5	6	8	No						1/8	NWC	0	4	No							DELAM NE CORNER 0 TO 4 4' H X 8" W
	8	5						1	6	8	No	-																
	10	5	3.2'					1.5	6	8	No						1/8	N	2	5	Yes							SCALE 1" DEEP ON CORNERS SCALE 1" DEEP ON CORNERS
	1	5		4	22	20	2	1	6	7	No																SE CORNER FH X 10'L X UP TO 2.5" D SPALL	SCALE 1" DEEP ON CORNERS
	2	5						1	6	7	No						3/16	W,N	0	4	Mar						WITH STIRRUPS AND REINF. EXPOSED 40% SI ON BAR: SCALE UP TO 3/4" DEEP: SOUTH	
	4	5						1	6	7	No						3/8	S.W.E.N	0	4	Yes						FACE BELOW P4 SPALL 3.5' W X 2' H X 4" D	
9																											BELOW P5 S FACE SCALE 2" DEEP; WEST	
	5	5	3.9'					1	6	7	No						1/16	s	0	3	No						NOSE 2' HIGH X 3' WIDE X 4" DEEP REINF EXPOSED 40% SI : NORTH FACE: ISO AREAS	
																											AT PILES 3,4,5 SCALE 1.5" DEEP	
	1	5		3.5				_									HL	NEC	0	2		Y	6	8.5	BAG		EAST FACE: FH X FW X 3.5" DEEP REINF	FAILED BAG REPAIR TOP 3*
	2	5						_				_					1/8	N	0	4		Y	6	8.5	BAG		EXPOSED 40% SL SOUTH FACE: FL X 2'H X 3" DEEP SCALE WITH RANDOM REINF	DELAM 0 TO 4, 4'H X 10" W SEC
10	4	5			22	22	3															Y	6	8.5	BAG		EXPOSED 25% SL WITH ADJACENT DELAM	
	-	E	4.4														1/16	EN	0	2	Yos	×	e	9.5	RAC		SCALE 3.5" DEEP WITH 100% SL ON	
	5	0																2,0	ů	0	105		0	0.0	0.00		STIRRUPS NORTH FACE SAME AS SOUTH	
																				-								
	2	5.25		0													1/4	S.W.E.N	0	3	Yes	Y	6	11	BAG			DELAM SOUTH FACE 2' DIA P=1
18	3	5.25															1/16	N,W	0	4	Yes	Y	6	11	BAG		SOUTH FACE: SCALE UP 12 TO 15" DEEP HARDWARE GONE NORTH FACE SAME	
	4	5.25																				Y	6	11	BAG			
	5	5.25	7		22	22	7										1/4	W,S	0	6	Yes	Y	6	11	BAG			NEC DELAM 2'H X 10" W
	1	5.25		11	22	22	11										1/8	N.S.W	0	3	No	Y	3	8	SQ EPOXY			1.5" ANNULUS ON JACKET
	2	5.25										0.75	SWC	3	3		1/4	W, N	0	3	Yes	Y	3	8	SQ EPOXY			SPALL 6" DIA X 3/4" D
19	3	5.25															1/16	S,E,N	0	2	Yes	Y	2	9	BAG		NO STRUT HERE	
	4	5.25	10					-									1/8	W,S,E,N	0	3	Yes	Y	3	6	SQ EPOXY			WITH ASSOCIATED SPALLING 3"W X 1.5" D
	5	5.25	10														1/0	E,14,44	Ū	3	Tes		3	0	SQ EPOXI			
	1	5.25		11													1/16	E,N	0	4	Yes	Y	6	13	BAG			
	2	5.25			22	22	12										1/8	W,N	0	4	Yes	Y	6	12	BAG			
20	3	5.25						-									1/16	E	12	17	No	Y	6	12	BAG		INTERMITTENT AREAS FD, STIRRUPS 100% SL	VC SOUTH FACE 1/16" W 0 TO 3
	4	5.25															1/4	IN,S	0	4	Tes	T	0	12	BAG		MAIN REINFORCING 75 TO 90% SL	
	5	5.25	11														1/8	W,N	0	4	Yes	Y	6	12	BAG			ASSOCIATED DELAM AND SPALLING 4"W X 2"D
	1	5.5		10								2	S, NWC	0	4		1/4	S,W	0	4		Y	6	11	BAG			8"H X 6"W X 2.5" D NWC SPALL
	2	5.5										0.25	N	0	4		>= 1/2	W,S	0	2	Yes	Y	6	13	BAG		UP TO 2" INTERMITTENT STRUT EXPOSED 25%	NORTH FACE SHALLOW SPALLING; CRACKS HAVE ASSOC, HONEYCOMBING
28	3	5.5						1							1	1	1					Y	6	13	BAG	1	SL WEST END: SPALLING/SCALE 10'L X FD (10") DEBONDED REINR WITH UP TO 100%	
	4	5.5			22	22	11															Y	6	13	BAG		SL EAST NOSE: 12L X FH X FD	
	5	5.5	11														1/16	SF	0	4	No	Y	6	13	BAG		SCALE/SPALLING WITH MULTIPLE REINF AND STIRRUPS EXPOSED WITH 100% SL	
	, i i i i i i i i i i i i i i i i i i i																	-,-	-									
				15															-	-			-					
	2	5.5		10					-		+						1/16 3/8	E,N,S S.NWC	0	2	No Yes	Y	6	12	BAG		NORTH AND SOUTH FACES: BOTTOM 41 200	
	3	5.5			22	22	9	1							1	1	3/8	NWC	0	4	Yes	Y	6	11	BAG	1	SCALE INTERMITTENT STRUT EXPOSED	CRACK WITH ASSOCIATED SPALLING 6" DIA X
29	4	5.5						-		-	+		+		+	+	1/16		0	3	Vee	· ·	6	12	BAG		EAST NOSE: FH X FL X 8" DEEP SCALE WITH EXPOSED REINF 80% TO 100% SL WEST	DEEP
	4	5.5	40				-	1.									1/16	5	0	3	Tes	T		12	DAG	l	NOSE SAME AS EAST	ABOVE BAG REPAIR UP TO 1" DEEP SCALE AT
	5	5.5	10					1	6	/							3/8	E,SWC	U	4	Yes	Ŷ		12	BAG			CORNERS

																Defect Co	ond t on Data											
		nventory Da	ta			Leve Dat	a		Abras	on Sca e				Spa				١	Vert ca Crack	ng			Ex s tri	g Repa rs		Manua		
Ben No	P e No	Top o WL ( )	Wes End Ben ML Deph()	Eas End Ben ML Deph()	Org Xse (n)	ec M n X Sec (n)	ML Dep h (	) Dep h (	(n) Deec Top	De ec Bo om	Exp Stee	Dep h ( n)	Face	De ec Top	De ec Bo om	Exp Stee	W d h (n)	Face	De ec Top	De ec Boom	Rus Stan	Yes No	Repar Top	Repar Boom	Repar Type	Damage Ra ng	Concrete Strut Notes	Add t ona Notes
	1	5.5	WI TO MI	11 WI TO MI	22	22	11															Y	6	12	BAG			
	2	5.5						1	6	7												Y	7	12	BAG		2* DE SCALE STIRRUPS EXPOSED AND	
30	4	5.5															1/16	S,E,N	0	4	Yes	Y	6	12	BAG		WIDE NEAR THE TOP EDGE EAST NOSE: FF	
	5	5.5	11														1/8	N,W,S	0	4	Yes	Y	6	12	BAG		X 6" L X 4" DEEP SCALE EXPOSING STIRRUPS WITH 50% SL	WEST FACE ASSOCIATED SPALLING 2" DIA 3 2"D; NE CORNER AND EAST FACE HC 6" DIA X D
	1	6.25		14	22	22	14					0.5	NW	3	4		1/4	S, E, N	0	7	Yes	Y	6	12	BAG			NE CORNER, CRACK WITH ADJACENT SPALLIN SHALLOW SPALL ON NORTH FACE 1' HGIH 0. WIDE 1/2* DEEP.
	2	6.25						_														Y	7	13	BAG		SOUTH FACE, FULL LENGTH X FULL HEIGHT > 5" DEEP SPALLING WITH EXPOSED REBAR	
20	3	6.25															1/4	S, N	0	4	Yes	Ŷ	7	13	BAG		STIRRUPS HAVE 100% SECTION LOSS. EAST	2 CRACKS ON SOUTH FACE.
38	4	6.25										0.5	N	3	3.5							Y	7	13	BAG		HISSING. NORTH FACE FULL LENGTH X FULL HEIGHT X 5" DEEP SPALL WITH EXPOSED REBAR, STIRRUPS 100% SECTION LOSS.	TYP BAG NOTE: TOP 10% OF FABRIC IS WEARING AWAY BUT GROUT UNDERNEATH I OK. SE CORNER LIGHT HONEY COMBING ABO STRUT.
	5	6.25	15									1.5	NW	2	4	NO	1/8	S, NW	0	6	Yes	Y	Y	13	BAG			NORTH WEST CORNER HAS CRACKS WITH ASSOCIATED SPALLING HAIRLINE CRACKS O NORTH FACE (0 TO 5).
																												VC ON S. E. N AND W. FACES 0 TO 5 UP TO 2/
	1	6.25		15								0.25	s	3	4	NO	3/16	ALL	0	6	YES	Y	7	13	BAG			WIDE WITH RUST STAINING. N FACE VC 1/4" WIDE WITH RUST STAINING. NW CORNER HA LIGHT SPALLING AND DELAM (1 TO 4) 4" WIDE 3/4" DEEP.
20	2	6.25															3/16	s	0	4	YES	Y	7	13	BAG		EAST END OF STRUT MISSING. SOUTH AND NORTH FACE HEAVY SPALLING AND SCALE FULL HEIGHT X FULL LENGTH UP TO 6" DEEP	VC ON S (0 TO 4). N CORNER CRACKS UP TO 1 WIDE. NW CORNER SCALE AND DELAM (1 TO 8" WIDE X 1" DEEP.
39	3	6.25																				Y	7	13	BAG		LOSS, LONGITUDINAL BARS DEBONDED AND	
	4	6.25										o	w	o	2	NO	1/8	SE	0	4	YES	Y	7	13	BAG		100% SECTION LOSS. WEST END OF STRUT MISSING.	W FACE DELAM (0 TO 2) 1' HIGH X 6" WIDE. I FACE VERTICAL CRACKS (0 TO 6) WITH ASSOCIATED SPALLING UP TO 1/4" WIDE CRACKS.
	5	6.25	14		22	22	14		2	3	NO						3/16	N, E, W	0	4	YES	Y	7	13	BAG			SCALE ON NW CORNER 1' HIGH 3" WIDE 1" DEEP.
	1	6.25		14													1/4	SW, SE	0	4	YES	Y	7	13	BAG		-	BAG REPAIR: AT TOP OF BAG AT THE SE CORNER, REBAR HAS DEBONDED FROM THI GROUTH 1' HIGH X 4" WIDE X 1.5" DEEP.
	2	6.25																				Y	7	13	BAG			
	4	6.25																				Y	7	13	BAG		FULL LENGTH X 6" DEEP. BOTTOM 1' HAS ADDITIONAL SCALING 2" DEEP, STIRRUPS AR	BAG REPAIR: TOP OF BAG REPAIR HAS A VOID
40	5	6.25	15					1.5	5 2	4	NO						1/4	s	0	4	NO	Y	7	13	BAG		EXPOSED WITH 100% SECTION LOSS, REINFORCING ALSO HAS 100% SECTION LOSS. EAST END OF STRUT TOP 1' HAS HEAV	NE CONER HAS SCALE 2 CRACKS ON S FAC
	6	6.25																				Y	7	13	BAG		SPALLING 1' HIGH X FULL WIDTH X FULL DEPTH (8" DEEP). EAST FACE HEAVY SPALL	
	7	6.25		_	_			_														Y	7	13	BAG		AND SCALE TOP 1'-8" DEEP.	
	8	6.25															3/16	N	0	4	YES	Y Y	7	13	BAG		-	
	10	6.25			22	22	15					2	NW	2	3	NO	1/4	NW	0	5	NO	Y	7	13	BAG			VERTICAL CRACKS ON N FACE AND ASSOCIATED SPALL 8" WIDE.
	1	6.5		14													1/8	S, W	0	5	YES	Y	7	13	BAG			HAIRLINE CRACKS ON W FACE.
	2	6.5																				Y	7	13	BAG		SOUTH FACE SCALE/SPALLING FULL LENGTH	
	3	6.5			22	22	15					-					3/16	S, W	0	4	YES	Y	7	13	BAG		X FULL HEIGHT, WORSE ALONG BOTTOM 2' UI TO 8" DEEP. TOP OF STRUT ON SOUTH FACE AT PILES 1 AND 3. STIRRUPS 100% SECTION	W FACE HAS A 1/8" WIDE DIAGONAL CRACK WITH ASSOCIATED 1" DIA DELAM.
48	4	6.5	15			-				1	1	1	1	1			1/4	NW, SW	0	4	YES	Y	7	13	BAG		LOSS AND REINFORCING 50% SECTION LOSS WEST AND EAST ENDS OF THE STRUT HAVE SPALLING UP TO 8" DEEP STIRRUPS AND	SW AND NW CORNERS HAVE CRACKS AND DELAM UP TO 1/4" WITH RUST STAINING. DEL
	6	6.5			-		-	-						-			1/16	N W	n	4		Y	7	13	BAG		REINFORCING 100% SECTION LOSS. NORTH FACE HAS SCALE SPALLING HAS FULL	4" H X 10" WIDE.
	7	6.5																		~		Y	7	13	BAG		LENGTH X FULL HIEGHT UP TO 4" DEEP WITH INTERMITTENT AREAS OF REINFORCMENT	
	8	6.5															3/16	NW	0	4	No	Y	7	13	BAG		EXPOSED. AT PILE 4, NORTH FACE HEAVY	
	9	6.5		+				+						-			1/4	NW	0	4	Y	Y	7	13	BAG		SPALLING 8" DEEP.	CRACK ON N FACE HAS ASSOCIATED DELAN
	10	0.0															1/4	1997	0				,	15	DAG			GIGIGIC ON IN PAGE THIS ASSOCIATED DELAN

																Defect Co	nd t on Data											
		nventory Da	ita			Leve Data			Abras	on Sca e				Spa				,	Vert ca Crack	ng			Ex s tn	g Repa rs		Manua		
Ben No	P e No	Top o WL ( )	Wes End Ben ML Deph()	Eas End Ben ML Deph()	Org Xsec (n)	M n X Sec (n)	ML Dep h()	) Dep h ( n)	Delec Top	De ec Boom	Exp Stee	Dep h (n)	Face	De ec Top	De ec Boom	Exp Stee	W d h (n)	Face	De ec Top	Deec Boom	Rus Stan	Yes No	Repa r Top	Repar Boom	Repar Type	Damage Ra ng	Concrete Strut Notes	Add t ona Notes
	1	6.5	WI IOM	13	22	22	13					2	NE	1	2		1/8	S, E	0	4	NO	Y	7	13	BAG		EAST END HAS HEAVY SPALLING, TOP 1' UP TO 8" DEEP WITH EXPOSED AND DEBONDED	SPALL ON NE CORNER 1' H X 10" WIDE X 2" DEEP.
	2	6.5							-								1/8	S, E	0	4	NO	Y	7	13	BAG		REINFORCEMENT WITH 100% SECTION LOSS. SOUTH FACE HAS SCALING/SPALLING FULL	
49	4	6.5															1/0	3, E, W, N	0	~	NO	Y	7	13	BAG		LENGTH X FULL HIEGHT UP TO 6" DEEP ALONG THE BOTTOM 2 FEET WEST END OF STRUT	
	5	6.5	14									1.5	NW	2	4	NO	1/4	w	0	5	Yes	Y	7	13	BAG		100% MISSING. NORTH FACE HAS HEAVY SPALLING UP TO 6" DEEP FULL HEIGHT X FULL LENGTH.	W FACE HAS ASSOCIATED SPALLING AT NW CRACK,
	1	6.5		15													1/8	S, E	0	4	YES	Y	7	13	BAG		EAST END HAS HORIZONTAL CRACK 8' LONG	
50	3	6.5																				Y	7	13	BAG		WITH ASSOCIATED DELAM S FACE HAS ABRASION UP TO 3 INCHES DEEP NO REBAR	
	4	6.5																				Y	7	13	BAG		EXPOSED. SAME ON NORTH FACE. WEST END SCALE UP TO 2" DEEP NO REBAR	
	5	6.5	15		22	22	13										1/8	S,, N FACE	0	4	YES	Y	7	13	BAG		SCALE OF TO 2 DEEP NO REBAR.	
		4.5																										
		4.5																										NWWW: footing (2' thick) exposed FL x 2'-0"H. scal
		4.5																										spall at waterline. Abutment stem: -at west end, 2
N. Abutme	nt	4.6																										0"H x 1'-0"W x 3"D spall. Footing exposed FL x up 1 3'-0"H with scale up to 3" deep throughout. 6'-0"H x
		4.5																										0"w x 2"D spall at centerline and similar spall at east and NEWW footing exposed EL x 2-5"H with scale
		4.5																										up to
															-													
	1	4.5		5.5								4	E	4	'	Yes	1/4	E	U	4	Yes	Ŷ	'	10	BAG			
150	2	4.5			22	22	5.5										1/4	S,E	0	4	Yes	Y	7	10	BAG		FULL DEPTH SCALE/SPALLING. STIRRUPS	
	3	4.5															1/16	N	1	4	Voc	Y	7	10	BAG		HAVE 100% SL	
	5	4.5	5.5									5	w	4	7	Yes	1/4	S,W	1	4	Yes	Y	7	10	BAG			
	1	4.5		5.5								4	SE	0	5	Yes w/ SL	1/8	N,E,S	0	4	Yes	Y	7	10	BAG			
	2	4.5							-								1/4	WS	0	4	Vee	×	7	10	BAG			
149	3	4.5			22	22	5.5										1/4	11,0	Ū		103	Y	7	10	BAG		FULL DEPTH SCALE/SPALLING. STIRRUPS	
	4	4.5															1/4	N,W	0	4	Yes	Y	7	10	BAG		HAVE 100% SL	
	5	4.5	5.5									6	w	3	6	Yes w/ SL	1/4	W,N,S	0	4	Yes	Y	7	10	BAG			FULL WIDTH SPALL WITH FULL REBAR CAGE
																												BROOLD
	1	4.5		6								3	F	4	6	Vee	1/4	EN	0	4	Ves	~	7	10	BAG			
		4.0		0								Ů	-			105		2,14	°.		103			10	5.00			
148	2	4.5			22	22	6						NW, SW	0	4		1/4	N,W,S	0	4	Yes	Y	7	10	BAG		ALL FACES FULL LENGTH X FULL HEIGHT X FULL DEPTH SCALE/SPALLING STIRRUPS	DELAM WITH RUST STAINING
	5	4.5			- 22	22	0																,	10	BAG		HAVE 100% SL	ABRASION WHERE STRUT IS MISSING REBAR I
	4	4.5						2	5	6	Yes						1/8	W	0	4		Ŷ	7	10	BAG			EXPOSED
	5	4.5	6									4	N,W,S	3	7	Yes w/ SL	3/16	N,W	0	4		Y	7	10	BAG			
	1	4.5		11													1/4	NW F	0	4		Y	4	10	SO EPOXY			ASSOC DELAM WITH CRACKS
	2	4.5															1/8	ALL	0	4	Yes	Y	4	10	SQ EPOXY			
131	3	4.5															1/4	ALL	0	4	Yes	Y	4	10	SQ EPOXY		NO STRUT THIS BENT	
	4	4.5			22	22	11										1/4	ALL	0	4	Yes	Y	4	10	SQ EPOXY			
	5	4.5	11														1/4	N,W,S	0	3		Y	4	10	SQ EPOXY			JACKET MISSING ON S. FACE WITH 1" OF SCAL ON GROUT
	1	4.5		12								1.5	SW	2	3		3/16	ALL	0	4		Y	4	10	SQ EPOXY			JACKET IS DAMAGED FROM 4 TO 7
	2	4.5			22	22	10	1.5	7	9		1				1	1/4	ALL	0	5		Y	4	10	SQ EPOXY			JACKET IS MISSING FROM 4 TO 9 SCALE ON PILE
130	3	4.5						1.5	7	9		1.5	SW	3	4		1/4	ALL	0	5		Y	4	10	SQ EPOXY		NO STRUT THIS BENT	JACKET IS MISSING FROM 4 TO 9. SCALE ON PILE
	4	4.5															1/4	ALL	0	4		Y	4	10	SQ EPOXY			JACKET DAMAGED FROM 4 TO 7
	5	4.5	12														1/4	N,S	0	4	Yes	Y	4	10	SQ EPOXY			
	1	4.5		14													1/4	ALL	0	4		Y	4	10	SQ EPOXY			
	2	4.5															1/4	ALL	0	4	YES	Y	4	10	SQ EPOXY		]	
129	3	4.5		_	22	22	12			1		1.5	NW,SW	3	4		3/16	ALL	0	4	YES	Y	4	10	SQ EPOXY		NO STRUT THIS BENT	
	4	4.5	13						+	+	1	1.5	NW	1	4	+	1/4	S,W,N	0	4	Yes	Y	4	10	SQ EPOXY		1	
1	1 0	4.0	10														<ul> <li>1/9</li> </ul>					• T	. 4	. 10	, JU CHUAY		-	

																Defect Co	nd t on Data											
		nventory Dat	8			Leve Data			Abras o	on Sca e				Spa				v	ert ca Crack i	ng			Ex s tng	Repa rs				
Ben No	P e No	Top o WL ( )	Wes End Ben ML Deph() WI TO MI	Eas End Ben ML Deph() WI TO MI	Org X sec (n)	M n X Sec (n)	ML Dep h()	Deph(n)	De ec Top	De ec Boom	Exp Stee	Dep h ( n)	Face	De ec Top	De ec Boom	Exp Stee	W d h (n)	Face	De ec Top	De ec Boom	Rus Stan	Yes No	Repa r Top	Repar Boom	Repar Type	Damage Rang	Concrete Strut Notes	Add t ona Notes
	1	4.5		12													3/8	N,W,S	0	4	YES	Y	4	10	SQ EPOXY		-	
128	2	4.5										1	NDM	2	4	DC	3/16	S	0	4	Yes	Y	4	10	SQ EPOXY		NO STRUT THIS BENT	
120	4	4.5			22	22	11						INVY	2	4	RS	1/8	S.W	0	4	Yes	Y	4	10	SQ EPOXY SQ EPOXY		NO SIKOT INIS BENT	
	5	4.5	13														1/4	E,S	0	4	YES	Y	4	10	SQ EPOXY		1	
										-													_					
		4.0		14				0.5	4								3/10	5	U	4			'	13	BAG			
	2	4.5			22	22	14															Y	7	13	BAG			
	3	4.5															1/4	W,S	0	4	YES	Y	7	13	BAG			ASSOC SPALLING WITH CRACK
	4	4.5															1/4	W,E,S	0	4	YES	Y	7	13	BAG		EAST AND WEST ENDS: STRUT MISSING	
111	5	4.5		14				1	4	7							1/4	N,S	0	4	YES	Y	7	13	BAG		FULL HEIGHT X FULL DEPTH SCALE/SPALLING.	
	6	4.5			22	22	14	1	6	7							3/8	N	0	4		Y	7	13	BAG		STIRRUPS HAVE 100% SL	
																							-				4	ASSOC DELAM/IMMINENT SPALL NW CORNER
	/ •	4.5															1/8	N,W	0	4	YES	Y	7	13	BAG		4	
	9	4.5															3/16	W.E	0	4	YES	Y	7	13	BAG		-	
	10	4.5	15														1/4	N,S	0	4	Yes	Y	7	13	BAG		1	
	1	5		14				0.5	4	7		2	SW	0	4		1/2	ALL	0	4	YES	Y	7	13	BAG		FAST AND WEST ENDS: STRUT MISSING	
110	2	5																				v	7	13	BAG		NORTH AND SOUTH FACES: FULL LENGTH X	
110	3	5															1/8	S	0	4	YES	Y	7	13	BAG		FULL HEIGHT X FULL DEPTH SCALE/SPALLING. STIRRUPS HAVE 100% SI	
	4	5			22	22	15										1/4	N,W,S	0	4	YES	Y	7	15	BAG			
	5	5	15					1.5	4	7		3	SW	3	7		1/4	S,N				Y	7	13	BAG			
	1	5		14													3/8	N,S,	0	4	Yes	Y	7	13	BAG		EAST AND WEST ENDS: STRUT MISSING	
100																											NORTH AND SOUTH FACES: FULL LENGTH X	
109	2	5																				Y	7	13	BAG		STIRRUPS HAVE 100% SL RANDOMLY	
	3	5															1/16	147	0	4	VEC	Y	7	13	BAG		EXPOSED	
	5	5	13		22	22	13	2	4	7		4	NW.SW	0	4	Yes w/ SL	1/10		0	-	120	Y	7	13	BAG			
	1	5		14				0.5	4	7							3/16	ALL	0	4	Yes	Y	7	13	BAG		NORTH AND SOUTH FACES: FULL LENGTH X	
																											FULL HEIGHT X 4"D SCALE/SPALLING.	
108	2	5																				Y	7	13	BAG		STIRRUPS HAVE 100% SL RANDOMLY EXPOSED EAST AND WEST ENDS: STRUT	
	3	5															3/8	N,W	0	4		Y	7	13	BAG		MISSING	
	4	5	45		22	22	13	-		-		2	NW,SW	0	4		3/8	N,S,W	0	4	Yes	Y	7	13	BAG		4	
	5	5	15						4	/							3/16	N,W,S	0	4	TES	T	/	13	BAG			
1	1	5		19		1					1	5	S,E,N	3	7	Yes w/ SL	1/4	ALL	0	4	Yes	Y	7	13	BAG	1	FACT AND WEAT OLDER, ATOLIATE WARMER	SPALLS REBAR CAGE EXPOSED DEBONDED REBAR WITH 50% SL
1	2	5		1	22	22	19	1	1		1	1			I	1						Y	7	13	BAG	1	NORTH AND SOUTH FACES: FULL LENGTH X	
90	3	5							1	İ			İ				1/4	NW	0	4	Vee	v	7	13	BAG	1	FULL HEIGHT X 4*D SCALE/SPALLING.	EAST FACE ASSOCIATED SPALLING 4" WIDE X 2"
1	5			+		+			I		1						1/4	14,77			105	· ·	'	13	DAG	I	EXPOSED	DEEP WITH RUST STAINING
	4	5										-		-			1/8	S,W,N	0	4	Yes	Y	7	13	BAG		1	
	5	5	18									/	W,N,S	3		Yes w/ SL	1/8	N,S,W	U	4	Yes	Ŷ	/	13	BAG	sv		SFALL REDAR GAGE EAPOSED 50% SL
	1	5		19				1.5	6	7	No						1/8	ALL	0	4	Yes	Y	7	13	BAG			
1	2	5								1				1							1	Y	7	13	BAG	1	EAST AND WEST SIDE: MISSING NORTH	
89	3	5															1/4	ALL	0	4	Yes	Y	7	13	BAG		AND SOUTH FACES: FULL LENGTH X FULL	CRACKS ASSOCIATED DELAM
1	4	5		1	22	22	16					DELAM	SEC	0	4		1/4	N,E,S	0	4	Yes	Y	7	13	BAG		HEIGHT X 4"D SCALE/SPALLING	DELAM/IMMINENT SPALL 4' HIGH X 8" WIDE
	5	5	19	-		-		1.5	4	7	No					-	1/4	W,E,S	0	4	Yes	Y	7	13	BAG	1		SWC CRACK SPALLING 4" WIDE X 2.5" DEEP
	1	E		19													1/4	ALL	0	4	Yee	v	7	12	BAC			CRACKS HAVE ASSOCIATED SPALLING 25 DIA
1	2	5	l	10	22	22	18									+	1/4	NW	0	4	Yes	T Y	7	13	BAG	t	EAST AND WEST SIDES: 4" DEEP SCALE	GIVENS THE ASSOCIATED SPALEING 2 DIA
88	3	5		1			10		1		1	0.75	N	2	3	No	1/8	W,N	0	4	Yes	Ý	7	13	BAG	1	OVER FULL LENGTH NORTH AND SOUTH	SPALL 1' HIGH X 6" WIDE X 3/4" DEEP
1	4	5							1								1/8	N,E	0	4	Yes	Y	7	13	BAG	1	SCALE/SPALLING	
	5	5	18									1.5	W	2	3	No	>= 1/2	W,S,E	0	4	Yes	Y	7	13	BAG			NWC SPALL 5" DIA X 3/4" DEEP

																Defect Co	nd t on Data											
		nventory Da	ta			Leve Dat	a		Abras	on Sca e				Spa					Vert ca Crack	ng			Ex s th	g Repa rs		Manua		
Ben No		Top o WL ( )	Wes End Ben ML Deph() WITOMI	Eas End Ben ML Deph() WITOMI	Org X sec (n)	M n X Sec (n)	ML Dep h (	) Dep h ( n)	De ec Top	Deec Boom	Exp Stee	Dep h ( n)	Face	De ec Top	De ec Bo om	Exp Stee	W d h (n)	Face	De ec Top	Deec Boom	Rus Stan	Yes No	Repar Top	Repar Boom	Repar Type	Damage Rang	Concrete Strut Notes	Add t ona Notes
	1	5		18				0.5	6	7	No	4	S/E FACE	S 3	7	Yes w/ SL	>= 1/2	ALL	0	3	Yes	Y	7	13	BAG			25% SL
	2	5															1/8	N	0	4	Yes	Y	7	13	BAG			
	3	5						0.5	6	7	No						1/16	W,SEC	0	4	Yes	Y	7	13	BAG			
	4	5						0.5	6	7	No						1/8	W,S,N	0	4	Yes	Y	7	13	BAG		WEST AND EAST SIDES: COMPLETIES	
71	5	5			22	21	17	0.5	6	7	No						1/4	NWC,S	0	4	Yes	Y	7	13	BAG		GONENORTH AND SOUTH FACES: FULL	CRX WITH ASSOCIATED SPALLING 3" WIDE 3 1.5" DEEP
	6	5		18				0.5	6	7	No						1/8	E	0	4	Yes	Y	7	13	BAG		SCALE/SPALLING	
	7	5						0.5	6	7	No						1/8	E,N	0	4	Yes	Y	7	13	BAG			
	8	5			22	21	16	0.5	6	7	No						1/8	ALL	0	4	Yes	Y	7	13	BAG			ASSOCIATED SPALLING 2" WIDE X 2" DEEP
	9	5						0.5	6	7	No						1/4	N,W	0	4	Yes	Y	7	13	BAG			
	10	5	17					0.5	6	7	No						1/4	S,E,W	0	6	Yes	Y	7	13	BAG			ASSOCIATED SPALLING 2* WIDE X 2* DEEP
	1	5		19													1/8	E,S	0	4	Yes	Y	4	9	SQ EPOXY			
	2	5															1/4	N, SWC	0	4	Yes	Y	4	9	SQ EPOXY			ASSOCIATED SPALLING 8" HIGH X 3" WIDE X DEEP
70	3	5			22	22	18										1/4	W,S	0	4	Yes	Y	4	9	SQ EPOXY		NO STRUT HERE	
	4	5										1	NEC	2.5	4	No	1/8	ALL	0	4	Yes	Y	4	9	SQ EPOXY			1.5' HIGH X 8" WIDE X 1" DEEP SPALL
	5	5	20					1	4	6	No	0.75	NEC	0	2	No	1/8	S,W	0	4	Yes	Y	6	9	SQ EPOXY			MISSING JACKET ON SOUTH AND WEST FACE
	1	5		17													1/8	S,E,N	0	3	Yes	Y	3	9	SQ EPOXY			
	2	5															1/8	ALL	0	3	Yes	Y	3	9	SQ EPOXY			
69	3	5																				Y	3	9	SQ EPOXY		NO STRUT HERE	
	4	5			22	22	17										1/8	E,N,S	0	3	Yes	Y	3	9	SQ EPOXY		1	
	5	5	18														1/8	W	0	3	Yes	Y	3	9	SQ EPOXY			

Appendix B - Photos



Photo 1: Overall of the South Half of the Pier (East Elevation).



Photo 2: Overall of the North Half of the Pier (East Elevation).





Photo 3: Typical 5-Pile Bent (Bent 8, North Elevation Shown).



Photo 4: Typical 5-Pile Bent (Bent 10, South Elevation Shown).





Photo 5: Typical Bent without a Reinforced Concrete Strut (Bent 129 Shown).



Photo 6: Typical 10-Pile Double Bent (Bent 8, North Elevation Shown).





Photo 7: South Elevation of Pier Wall 1 (Bent 56).



Photo 8: North Elevation of Pier Wall 6 (Bent 57).





Photo 9: North Abutment Elevation Looking Northeast.



Photo 10: Typical Spalling and Exposed Reinforcement on Strut (Bent 149 Shown).





Photo 11: Typical Spalling and Exposed Reinforcement on Strut (Bent 147 Shown).



Photo 12: Typical Spalling, Exposed Reinforcement, and Missing West and East Ends on Strut (Bent 111 Shown).





Photo 13: Typical Spalling and Exposed Reinforcement on Strut (Bent 28 Shown).



Photo 14: Typical Missing Strut on East and West Faces (Bent 39 Shown).





Photo 15: Typical Spalling and Exposed Reinforcement on West and East Ends of the Strut (Bent 30 Shown).



Photo 16: Typical Condition of Pile Underwater (Bent 30, Pile 1 Shown).





Photo 17: Bent 130, Pile 1, Corner Spall and Delamination.



Photo 18: Typical Vertical Cracking at the Top of Pile (Bent 150, Pile 5 Shown).





Photo 19: Bent 109, Pile 5, Area of Spalling and Delamination at the Top of Pile.



Photo 20: Bent 148, Pile 4, Area of Abrasion with Exposed Reinforcement (Underwater).





Photo 21: Bent 8, Pile 5, Corner Spall with Exposed and Corroded Reinforcement.



Photo 22: Bent 149, Pile 5, Spall with Exposed, Corroded, and Debonded Reinforcement.





Photo 23: Typical Condition of Square Epoxy Jacket Underwater (Bent 130, Pile 4 Shown).



Photo 24: Bent 130, Pile 2, Missing Jacket and Abrasion in the Tidal Zone.





Photo 25: Bent 130, Pile 2, Failed Square Epoxy Jacket (Underwater).



Photo 26: Bent 40, Pile 1, Void at the Top of the Grout Filled Fabric Bag Repair (Underwater).





Photo 27: Pier Wall 1 (Bent 56), Scaling and Voids at the Waterline.



Photo 28: Pier Wall 6 (Bent 57), Typical Void in Fiberglass Jacket at the Columns (Underwater).





Photo 29: Pier Wall 6 (Bent 57), Typical Condition of Reinforced Concrete Strut (Underwater).



Photo 30: Pier Wall 6 (Bent 57), Typical Condition of Footing (Underwater).





Photo 31: Pier Wall 6 (Bent 57), Void at Footing Subfooting Interface (Underwater).



Photo 32: Pier Wall 6 (Bent 57), Void in the Subfooting (Underwater).





Photo 33: North Abutment (Bent 151), Spalling at the Waterline.



Photo 34: North Abutment (Bent 151), Spalling at the Waterline.



Appendix C– ASCE Damage Rating Figure





Fig. 2-4. Damage ratings for reinforced concrete elements Source: Courtesy of CH2M HILL, Inc. and COWI, Inc., reproduced with permission.

38